Optimal parameters for the Green-Ampt infiltration model under rainfall conditions

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INTRODUCTION

The Green-Ampt (GA) model (Green and Ampt, 1911), and various modifications and extensions thereof, are used widely for analyzing relationships between rainfall, infiltration and runoff for a range of conditions and from different theoretical and empirical perspectives (Ahuja and Ross, 1973; Barry et al., 2005; Basha, 2011; Bouwer, 1978; Chen and Young, 2006; Gowdish and Muñoz-Carpena, 2009; Hilpert and Glantz, 2013; Hogarth et al., 2013; Mein and Larson, 1973; Parlange et al., 1982; Voller, 2011; among many others). Because of its simplicity and physical basis, the model has been implemented also in several codes addressing larger-scale hydrologic processes (e.g. Arnold et al., 2012; Flanagan and Nearig, 1995; O’Brien et al., 2009; Rossman, 2010).

As with most models, a critical issue in applying the GA parameterization is accurate estimation of the GA parameters. In current practice, GA model parameters are determined using both theoretical and empirical approaches (Neuman, 1976; Rawls et al., 1983; Rissie et al., 1994; Saxton and Rawls, 2006). A number of studies have compared the accuracy of alternative parameterizations against numerical solutions of the Richards (1931) equation for unsaturated or variably saturated flow, mostly for ponded infiltration scenarios (e.g. Barry et al., 1993; Hsu et al., 2002; Salvucci and Entekhabi, 1994). By comparison, the accuracy of the parameterizations has not been examined extensively for infiltration under rainfall conditions. Such studies are important since GA errors in estimating infiltration can magnify considerably when runoff estimates are needed, since runoff in many situations is a small fraction of rainfall. As compared to ponded infiltration, rainfall adds additional complexity to the accuracy of parameter estimation due to the transition from flux-controlled infiltration to profile-controlled infiltration. The objective of this study was to evaluate the performance of the GA model in comparison with the Richards equation, and to determine optimal GA parameters that would improve the utility of the model for simulating infiltration under rainfall conditions. We further studied the sensitivity of the optimal parameters to various rainfall intensities and durations.

THE GA INFILTRATION MODEL AND ITS PARAMETERS

Formulation of the GA model

The GA model assumes piston-type soil water movement and a step-function for the water content in the soil profile involving two water contents, the initial water content \( \theta_i \) and the saturated water content \( \theta_s \) near the soil surface as shown in Fig. 1.

Using Darcy’s law for these conditions, the infiltration rate \( i \) under ponded conditions is given by (Green and Ampt, 1911):

\[
i = \frac{dl}{dt} = K_s \left[ 1 + \left( \frac{\theta_s - \theta_i}{S} \right) \frac{S}{I} \right]
\]  

where \( i \) is time, \( I \) is the cumulative infiltration depth, \( K_s \) is the saturated soil hydraulic conductivity, and \( S \) is the capillary pressure at the wetting front, sometimes referred to as the capillary drive (e.g. Morel-Seytoux and Khanji, 1974) or macroscopic capillary length (Philip, 1985). In some studies (Rawls et al., 1983) an effective hydraulic conductivity \( K_e \) is used rather than \( K_s \) to account for hysteresis effects. This difference in interpretation will not affect the theoretical analysis in our work. Most studies directly use \( K_s \) in Eq. (6) for ponded infiltration studies (Mein and Larson, 1973; Morel-Seytoux and Khanji, 1974; Neuman, 1976). Our study also ignores the pond-
A number of analytical expressions for \( K(\psi) \) may be used for this purpose (e.g. Brooks and Corey, 1964; Kosugi, 1996; van Genuchten, 1980). Because of their complexity, most of these functions require numerical integration. An exception is the power law function of Brook and Corey (1964), which has been used in a number of GA applications, notably by Rawls and coworkers (Rawls et al., 1989, 1993). In our study we used the van Genuchten (1980) function for \( K(\psi) \) and integrated Eq. (6) numerically. Numerical integration may be too cumbersome for many practical studies. A useful approximation of \( S \) based on Eq. (6) for the \( K(\psi) \) function of van Genuchten (1980) is given by Morel-Seytoux et al. (1996).

\[
S = \frac{0.046m + 2.07m^2 + 19.5m^3}{(1 + 4.7m + 16m^2)\alpha}
\]  

where \( \alpha \) and \( m \) are parameters in the original van Genuchten-Mualem formulation (van Genuchten, 1980).

The use of analytical functions for \( K(\psi) \) has the advantage that many of the parameters in these functions are often immediately available, including through the use of pedotransfer functions which estimate the parameters from soil texture and related data (Rawls et al., 1983; Schaap et al., 2001; Vereecken et al., 2010). Alternatively, GA parameters could be estimated using field-based approaches to better account for site-specific conditions. Some studies aimed at deriving GA model parameters from field experiments, mainly focusing on the hydraulic conductivity. Single- and double-ring infiltrometer experiments (Nimmo et al., 2009; e.g. Stahr et al., 2004) and rainfall simulation (e.g. Risse et al., 1994) have been used for this purpose to back-calculate \( K_s \) from continuous measurements of the infiltration depth. Unfortunately, \( S \) cannot be determined easily using these approaches.

Most GA applications in the literature consider ponded infiltration, whereas many practical applications involve conditions in which the soil surface is initially unsaturated. Mein and Larson (1973) modified the model to approximate rainfall-infiltration cases using the ponded GA model equation before the onset of ponding. The appropriate choice of parameters determined for ponded infiltration conditions may be reduced further due to this approximation. In general, little attention has focused on the performance of the GA model when theoretical parameters are employed. Only Mein and Larson (1973) conducted limited tests to compare the theoretically parameterized GA model to the Richards equation.

An issue with most or all existing approaches is that model performance using estimated GA parameters is not quantified by comparisons to actual infiltration data or numerical predictions, or evaluated relative to other combinations of parameter values, especially under rainfall conditions. Therefore, infiltration simulation using the resulting parameters may not ensure optimal results. Prior work seeking optimal GA infiltration parameters is limited. To our knowledge, only Hsu et al. (2002) addressed this issue through comparisons with infiltration rates based on the Richards equation. However, as acknowledged in this paper, the GA model parameters include the saturated soil hydraulic conductivity \( K_s \), the capillary pressure at the wetting front \( S \), as well as the initial \( (\theta_i) \) and saturated \( (\theta_s) \) water contents. Of these, \( K_s \) is a relatively standard hydraulic parameter that can be measured in the laboratory or field using a variety of approaches. For ponded infiltration, \( S \) is given by (Morel-Seytoux and Khanji, 1974; Neuman, 1976):

\[
S = \frac{1}{K_s} \int_{-\infty}^{0} K(\psi) d\psi
\]
their paper, the approach was “less rigorous” in that GA parameters were determined by fitting to only three points rather than the entire infiltration curve. In addition, an artificial time shift parameter was used by Hsu et al. (2002). According to Eqs (3–5), this parameter is not independent since it can be calculated from other parameters.

The brief summary above of the GA model its parameters shows a need to systematically investigate performance of the model using existing parameterizations, as well as the need for practical guidance in seeking optimal GA model parameters, especially for infiltration under rainfall conditions.

**PERFORMANCE OF THE GA INFILTRATION MODEL**

In this study we combined the variables $S$ and $(\theta_s - \theta)$ in Eqs (1–5) into a single parameter, denoted as $MS$ (i.e., $MS = S(\theta_s - \theta)$). This combination is preferred since the effect of the initial soil moisture condition is then automatically taken into account for any $\theta_s$ value. Hereafter, we report on the two parameters $K_s$ and $MS$ for the GA model.

We used the established approach, first suggested by Mein and Larson (1973), for comparing GA simulation results with numerical solutions of the Richards equation to evaluate the use and accuracy of the GA parameterization. The Richards equation was solved using the van Genuchten (1980) soil hydraulic property relationships for $\theta(\psi)$ and $K(\psi)$. For the evaluation we numerically integrated Eq. (6) very carefully using Romberg integration, and multiplied the resulting $S$ values with $(\theta_s - \theta)$ to obtain $MS$.

Table 1 provides van Genuchten soil hydraulic parameters for the sand, sandy clay loam, and clay loam soils (Carsel and Parrish, 1988) used in the simulations. These soils were chosen to generate simulated infiltration data across a two-order-of-magnitude range in $K_s$. Fig. 2 shows the van Genuchten (1980) $\theta(\psi)$ and $K(\psi)$ relationships for these soils. Table 1 also shows the rainfall rates used in the calculations. We assumed constant rainfall intensities $p$, each lasting 10 hrs. To ensure that both infiltration and runoff covered a substantial part of total rainfall, rainfall intensities were set at approximately twice $K_s$ for all soils. The soil and rainfall data were used to compare GA infiltration results obtained with the Richards equation and the GA model. For example, for the sand soil, the GA model agreed closely with the Richards equation throughout the period of investigation. For the sandy clay loam soil, the GA model over-predicted the infiltration depth by up to 11% relative to the Richards equation. For the clay loam soil, the theoretical GA model parameters over-predicted the infiltration depth even more, this time by up to 25%. These results suggest that a better GA parameterization...
GA cumulative infiltration depths (i.e., to times in most applications. Also shown in the figures are comparisons, where the GA parameters is to calculate the overall error between the GA infiltration curve and the Richards equation for all possible $K_o$ and $MS$ parameter combinations, and then to identify those combinations corresponding to a minimum root mean squared error (RMSE). A similar approach was followed by Yin (2008). The RMSE was used as the objective function ($F$) to be minimized:

$$F = \sqrt{\frac{1}{k} \sum_{i=1}^{k} \left( \gamma_i - f(t_i, K_o, MS) \right)^2} \tag{8}$$

where $\gamma_i$ represents cumulative infiltration depths corresponding to times $t_i$ obtained with the Richards equation, $f(t_i, K_o, MS)$ are GA cumulative infiltration depths (i.e., $I$ in Eqs (1) through (3)) for parameters $K_o$ and $MS$, and $k$ is the number of samples for each infiltration curve. The objective function thus serves as an indicator of the overall accuracy of the GA model compared to the theoretical value.

The results in Figs 4–6 reveal several interesting features of the objective function. Most importantly, $F*$ for all soils converged to single minima that can be located on the plots. This indicates that optimal GA parameters exist and can be obtained using the optimization analysis. The GA simulation results with optimal parameters were notably better than those generated using the theoretical parameters for especially the medium- and fine-textured soils (Fig. 3). Therefore, for practical applications, the standard (theoretical) parameterization can be replaced with an optimization approach. In addition, the results in Figs 4–6 show that $F*$ is more sensitive to $K_o$ than to $MS*$, thus indicating that $K_o$ is a more important parameter than $S$ in GA model applications (especially for coarser soils).

In our study, the optimal hydraulic conductivity ($K_o$) greatly improved the GA model for especially fine-textured soils. Still, the use of GA model inaccurately reflects the effects of capillary forces on infiltration of especially fine-textured soils. For such cases one may obtain multiple acceptable solutions with the optimization although the objective function still has only one global minimum.

Another significant aspect of our results relates to the difference between the theoretical and optimal values of $K_o$ and $MS$. The dimensionless optimal parameters for the three test cases are included in Table 2. The data indicate that for the coarse- and medium-textured soils (the sand and sandy clay loam) in our study, the optimal hydraulic conductivity ($K_o$) was relatively close to $K_o$ such that $K_o$ approached unity, while the optimal $MS$ was substantially smaller than the theoretical $MS$ value (i.e., $MS*<<1$). For the fine-textured (clay loam) soil, the optimal $K_o$ was about 40% of $K_o$, while $MS*$ was larger than the theoretical value of $MS$.

<table>
<thead>
<tr>
<th>Soil type</th>
<th>$K_o$ (mm hr$^{-1}$)</th>
<th>$\alpha$ (mm$^{-1}$)</th>
<th>$n$</th>
<th>$K_o$ (mm hr$^{-1}$)</th>
<th>$MS$ (mm)</th>
<th>$\rho$ (mm hr$^{-1}$)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Sand</td>
<td>297</td>
<td>1.45×10$^{-2}$</td>
<td>2.68</td>
<td>297</td>
<td>14.8</td>
<td>500</td>
</tr>
<tr>
<td>Sandy clay loam</td>
<td>13.1</td>
<td>5.90×10$^{-3}$</td>
<td>1.48</td>
<td>13.1</td>
<td>10.4</td>
<td>25</td>
</tr>
<tr>
<td>Clay loam</td>
<td>2.6</td>
<td>1.90×10$^{-3}$</td>
<td>1.31</td>
<td>2.6</td>
<td>17.3</td>
<td>5</td>
</tr>
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</table>

The plots of the dimensionless objective function $F$ for the sand, sandy clay loam, and clay loam soils are shown in Figs 4–6, respectively. $F*$ represents the average relative error between the GA model and the actual infiltration curve obtained with solutions of the Richards equation. Note that errors in the final infiltration amount may be significantly higher than the average error given by $F*$. Substantial computational effort was required to obtain each of these maps since hundreds of thousands of GA model solutions were generated for each case. To show a wide range of $F*$ values on the plots in Figs 4–6, the upper bounds on $F*$ were set to 0.1 (10%), which is higher than acceptable error in most applications. Also shown in the figures are comparisons of infiltration curves obtained with solutions of the Richards equation and the GA model using the optimal parameters.
Table 2. Sensitivity of optimal parameters $K_p$ and $MS_p$ to rainfall intensity $p$ and duration $t_r$.

<table>
<thead>
<tr>
<th>Case</th>
<th>Soil texture</th>
<th>Rainfall intensity $p$ (mm hr$^{-1}$)</th>
<th>Duration $t_r$ (hr)</th>
<th>$p^*$</th>
<th>$t_r^*$</th>
<th>Optimal parameters</th>
<th>Theoretical parameters</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>Sand</td>
<td>300</td>
<td>3</td>
<td>1.010</td>
<td>60.203</td>
<td>0.997 0.136</td>
<td>1 1</td>
</tr>
<tr>
<td>2</td>
<td>Sand</td>
<td>300</td>
<td>5</td>
<td>1.010</td>
<td>200.676</td>
<td>0.999 0.066</td>
<td>1 1</td>
</tr>
<tr>
<td>3</td>
<td>Sand</td>
<td>300</td>
<td>10</td>
<td>1.010</td>
<td>60.203</td>
<td>0.985 0.536</td>
<td>1 1</td>
</tr>
<tr>
<td>4</td>
<td>Sandy clay loam</td>
<td>500</td>
<td>3</td>
<td>1.684</td>
<td>100.338</td>
<td>0.991 0.473</td>
<td>1 1</td>
</tr>
<tr>
<td>5</td>
<td>Sandy clay loam</td>
<td>500</td>
<td>5</td>
<td>1.684</td>
<td>200.676</td>
<td>0.995 0.399</td>
<td>1 1</td>
</tr>
<tr>
<td>6</td>
<td>Sandy clay loam</td>
<td>500</td>
<td>10</td>
<td>1.684</td>
<td>60.203</td>
<td>0.997 0.136</td>
<td>1 1</td>
</tr>
<tr>
<td>7</td>
<td>Sandy clay loam</td>
<td>15</td>
<td>3</td>
<td>1.145</td>
<td>6.298</td>
<td>0.992 0.159</td>
<td>1 1</td>
</tr>
<tr>
<td>8</td>
<td>Sandy clay loam</td>
<td>15</td>
<td>5</td>
<td>1.145</td>
<td>12.596</td>
<td>1.008 0.117</td>
<td>1 1</td>
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<tr>
<td>9</td>
<td>Sandy clay loam</td>
<td>15</td>
<td>10</td>
<td>1.145</td>
<td>6.298</td>
<td>0.908 0.714</td>
<td>1 1</td>
</tr>
<tr>
<td>10</td>
<td>Sandy clay loam</td>
<td>25</td>
<td>3</td>
<td>1.908</td>
<td>3.779</td>
<td>0.855 0.881</td>
<td>1 1</td>
</tr>
<tr>
<td>11</td>
<td>Sandy clay loam</td>
<td>25</td>
<td>5</td>
<td>1.908</td>
<td>12.596</td>
<td>0.954 0.556</td>
<td>1 1</td>
</tr>
<tr>
<td>12</td>
<td>Sandy clay loam</td>
<td>25</td>
<td>10</td>
<td>1.908</td>
<td>6.298</td>
<td>0.840 1.122</td>
<td>1 1</td>
</tr>
<tr>
<td>13</td>
<td>Sandy clay loam</td>
<td>50</td>
<td>3</td>
<td>3.817</td>
<td>3.779</td>
<td>0.741 1.522</td>
<td>1 1</td>
</tr>
<tr>
<td>14</td>
<td>Sandy clay loam</td>
<td>50</td>
<td>5</td>
<td>3.817</td>
<td>12.596</td>
<td>0.924 0.798</td>
<td>1 1</td>
</tr>
<tr>
<td>15</td>
<td>Sandy clay loam</td>
<td>50</td>
<td>10</td>
<td>3.817</td>
<td>6.298</td>
<td>0.980 1.122</td>
<td>1 1</td>
</tr>
<tr>
<td>16</td>
<td>Clay loam</td>
<td>5</td>
<td>3</td>
<td>1.923</td>
<td>0.451</td>
<td>0.296 3.524</td>
<td>1 1</td>
</tr>
<tr>
<td>17</td>
<td>Clay loam</td>
<td>5</td>
<td>5</td>
<td>1.923</td>
<td>0.751</td>
<td>0.385 2.546</td>
<td>1 1</td>
</tr>
<tr>
<td>18</td>
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<td>5</td>
<td>10</td>
<td>1.923</td>
<td>1.503</td>
<td>0.419 2.254</td>
<td>1 1</td>
</tr>
<tr>
<td>19</td>
<td>Clay loam</td>
<td>10</td>
<td>3</td>
<td>3.846</td>
<td>0.451</td>
<td>0.481 1.876</td>
<td>1 1</td>
</tr>
<tr>
<td>20</td>
<td>Clay loam</td>
<td>10</td>
<td>5</td>
<td>3.846</td>
<td>0.751</td>
<td>0.439 2.112</td>
<td>1 1</td>
</tr>
<tr>
<td>21</td>
<td>Clay loam</td>
<td>10</td>
<td>10</td>
<td>3.846</td>
<td>1.503</td>
<td>0.439 2.105</td>
<td>1 1</td>
</tr>
</tbody>
</table>

$^1$Cases corresponding to Figs 4, 5, and 6.

$^2$Subscript * denotes non-dimensional parameters, $p^* = p/K_s$, $t_r^* = t_r/t_c$, $K^* = K_o/K_s$ and $M_{S*} = M_{So}/M_S$.

In view of the above results, we also studied the time scale that characterizes the driving forces in the system to better explain their impact on the objective function and the optimized values of $K_p$ and $MS_p$. Following Smith (2002), a characteristic time scale can be defined as:

$$t_c = \frac{(\theta_s - \theta_w) S}{K_s} \quad (9)$$

where $S$ is the capillary pressure of the wetting front as used previously. The time scale characterizes the dominant force driving infiltration. Smith (2002) showed that for ponded cases, gravitational forces dominate infiltration and that the infiltration rate approaches $K_s$ for $t$ values larger than about 10$t_c$, while gravitational forces are negligible when $t$ is much smaller than $t_c$. In the current study (Table 1), values of $t_c$ can be obtained easily for the three soils: $t_c = 0.05$, 0.79, and 6.6 hr for the sand, sandy clay loam, and clay loam soils, respectively. The two orders of magnitude difference in $t_c$ reflects the wide range in soil hydraulic conditions for the three soils.

If we use $t_r$ for rainfall duration, then the ratios of $t_r/t_c$ (i.e., the dimensionless rainfall duration, denoted further as $t_{r*}$ for a ten-hour event are 200, 12.7, and 1.5 for the three soils, respectively. This explains why the optimal value of $K_p$ is close to the theoretical $K_p$ value for the two coarser-textured soils when the rainfall event extends to 10 hrs. These results also indicate that for short-duration events (i.e., events with a $t_{r*} < 10$), the GA parameters that best describe the infiltration depth are considerably different from the theoretical values.

Figure 6a also suggests that small $t_{r*}$ values can result in a narrow valley in parameter space where the GA parameters produce very similar values of the objective function, with all parameters being far superior than the theoretical values. This result has practical meaning, especially for arid environments where runoff generating rainfall events are often very intense and of short duration, thus yielding small $t_{r*}$ values. For such short-duration cases, optimal parameter sets should perform far better than the GA theoretical parameters, thus providing more accurate predictions of runoff.

Sensitivity of optimal parameters to rainfall intensity and duration

We next conducted a sensitivity test to evaluate changes in the optimal parameters as a function of rainfall intensity and duration. Two or three rainfall intensities and three durations were tested for each soil type (Table 2). Optimal parameters for each case were identified directly from plots of the objective function ($F^*$). Our objective of these tests was to find out if optimal parameters applicable for the various scenarios exist and could be identified.

Results of the sensitivity analysis are summarized in Table 2 for a total of 21 cases we examined. The results and input conditions are provided in dimensionless form. They show that...
rainfall intensity and duration both affected the values of the optimal parameters. Optimal $K_r$ values were generally close to 1 (i.e., $K_r \approx K_s$) for long-duration events (large $t^*$), while optimal $K_r$ values were less than $K_s$, sometimes by up to 70%, for short-duration events (small $t^*$). Optimal $MS^*$ values were less than 1.0 for low rainfall intensities or longer durations, but larger than 1.0 for short duration events ($t^* < \sim10$). This again indicates that theoretical GA parameters are generally not the best choice for infiltration simulations, and hence that optimal parameters should be used. When $K_r$ was set equal to $K_s$ as a constraint in the optimization, acceptable $MS^*$ values were generally less than 1.

Optimal parameters as a function of rainfall duration and intensity showed some general trends. For example, $K_r$ generally increased with rainfall duration ($t^*$) and decreased with rainfall intensity ($p^*$), except for small $t^*$ values when $K_r$ increased with $p^*$. Conversely, $MS^*$ generally decreased with $t^*$ and increased with $p^*$, except for small $t^*$ values when $MS^*$ decreased with $p^*$. In addition, for long-duration rainfall events ($t^* > \sim10$), $K_r$ approached 1 and $MS^* < 1$, while for short-duration events ($t^* < \sim1.5$), $K_r < 0.5$ and $MS^* >1$. These results show that the GA model using theoretical parameters overestimated gravitational effects for small $t^*$, but underestimated these effects as $t^*$ increased. The optimal GA parameters compensated for such effects and could improve the simulations. In general, relatively large $t^*$ values affected the capillary component most, in which case that the optimal $MS^*$ changed more significantly as a function of rainfall intensity.

Figure 7 shows examples of $F^*$ distributions for different rainfall intensities and durations (Table 2). Cases for large $t^*$ values are not included since the optimal parameters did not change significantly for different rainfall conditions. Maps for medium and small $t^*$ values show $F^*$ values below 0.025 (i.e., overall errors were less than 2.5% of cumulative infiltration), which we consider acceptable for these type of simulations. Results indicate that overall performance of the GA model was affected substantially by rainfall conditions at medium $t^*$ values (Fig. 7a) where the $F^*$ distribution and optimal parameters changed significantly with rainfall conditions. For practical applications, optimal parameters for high rainfall intensities resulted in generally smaller errors for other rainfall conditions. This implies that optimal parameters are still applicable for the various conditions. For small $t^*$, the performance of the GA model was not significantly affected by rainfall conditions (Fig. 7b),
with optimal parameters for one case being immediately applicable to other cases. Table 3 shows the performance of the optimal parameters for clay loam soil cases included in Table 2. Results indicate that, in general, optimal parameters for high intensity rainfall events are more applicable to various conditions. This finding provides useful guidance for practical applications in terms of determining the most suitable GA parameters.

In real applications with complex rainfall conditions, the optimal parameters may be determined using as the objective function the average $F_*$ for all significant rainfall intensities, weighted by rainfall depth. In our study, however, such an approach was not followed since the weighing factors for various rainfall conditions cannot be determined without knowing specific rainfall characteristics. For this reason we provided only general recommendations.

CONCLUDING REMARKS

This study examined the general performance of the GA model for simulating infiltration under rainfall conditions by comparing GA calculations with numerical solutions of the Richards (a) Distribution of the objective function $F_*$ for the clay loam soil (including a close-up around the minimum; (b) Comparison of the optimal GA infiltration curve and solution of the Richards equation for the clay loam soil.

Fig. 6. (a) Distribution of the objective function $F_*$ for the clay loam soil (including a close-up around the minimum; (b) Comparison of the optimal GA infiltration curve and solution of the Richards equation for the clay loam soil.

Fig. 7. Comparison of $F_*$ distributions for different rainfall intensities and durations: (a) from bottom to top: cases 7, 11, and 15 in Table 2; (b) from bottom to top, cases 17, 19, and 21 in Table 2. equation. Our results demonstrate that standard (theoretical) values of the two GA parameters (i.e., $K_s$ and $M_s$) will introduce significant errors in rainfall-infiltration modeling for fine- to medium-textured soils assuming rainfall events with realistic durations. We showed that optimal parameters for the GA model do exist and can be used to greatly improve the accuracy of GA simulations.

Our results further show that optimal parameters differ from theoretical values and may be affected by the rainfall characteristics. In general, both rainfall intensity and duration changed the values of the optimal parameters, with duration having a more significant impact. For long-duration (large $t_r$) rainfall events, the optimal saturated conductivity ($K_o$) was close to $K_s$ (the theoretical input value), while the optimal $M_s$ value was much less than the theoretical $M_s$. For short-duration (small $t_r$) rainfall events, optimal $K_o$ values may become significantly smaller than $K_s$, whereas optimal $M_s$ will be much greater than the theoretical $M_s$. Whereas optimal parameters for one rainfall
condition may not perform satisfactorily for all other conditions, especially for medium $t_r$ values, parameters feasible for various rainfall conditions may still be identified by comparing errors for the different cases. A weighted average of errors can be used for complex rainfall conditions if rainfall characteristics and their frequencies are known. Optimal parameters for high intensity rainfalls were found to be generally more applicable to different rainfall conditions.

In practical applications, parameter values other than the optimal values still may be acceptable. For short-duration rainfall events (with likely more effects when soils are fine-textured), acceptable parameters were located in a narrow valley in $K_s$–$K_r$ parameter space, thus implying that these two parameters should be determined concurrently to ensure satisfactory predictions of infiltration. Applying the GA model actually implies using a GA soil to approximate the real soil. A common goal of infiltration modeling is to accurately predict cumulative infiltration. If this is the sole criterion for modeling performance, a GA soil may not necessarily bear the same physical properties as the real soil (e.g., $K_s \neq K_r$). For a specific application, different GA parameters may be acceptable to approximate the behavior of the real soil, although optimal parameters would still perform slightly better than other sets. This equivalinity of model parameters has been studied by Beven and Binley (1992) and Beven and Freer (2001), among others.

Practitioners may need to confront two situations when parameterizing the GA model. When a set of soil hydraulic parameters (such as the van Genuchten parameters or of other representations) are known, optimal parameters can be obtained simultaneously by fitting the GA model to the Richards equation (using the known soil hydraulic parameters). As a first approximation, pedotransfer functions could be used for this. However, if the soil hydraulic parameters are not known, and pedotransfer functions are judged to be too imprecise, infiltration experiments (preferably rainfall experiments) could be conducted to produce infiltration curves which then would replace the Richards equation in the optimization approach.

Finally, we note that the present analysis was carried out for ideal conditions involving a homogeneous soil profile. It may be interesting to conduct similar analyses for more realistic field conditions such as for layered soils, soils with sloping surfaces, or for macroporous (structured) soils exhibiting preferential flow paths. The approach followed in this paper is very much applicable also to those conditions. Our ultimate objective in all this was to obtain better estimates of infiltration for larger-scale hydrologic applications, including runoff estimation, for which numerical solutions of the Richards equation are still largely impractical.

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REFERENCES


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<th>Table 3. Normalized objective function $F_*$ for the clay loam soil assuming different parameters.</th>
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<td>Parameter values</td>
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<tr>
<td>$K_r$ MS</td>
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<tr>
<td>$L_r$</td>
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<tr>
<td>$p_*$</td>
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$F_*$ of the optimal parameter set for the corresponding rainfall condition.
Optimal parameters for the Green-Ampt infiltration model under rainfall conditions


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